## Lecture 6 <br> THE GEOMETRIC CHARACTERIZTIONS OF THE PLANE CROSS SECTIONS (continues) <br> Plan

1. The parallel - axis theorem for moment of inertia of a finite area.
2. Principal moments of inertia.
3. Solved problem.

### 6.1. The parallel - axis theorem for moment of inertia of a finite

 area.This quantity has the dimension of a length to the fourth power, perhaps in ${ }^{4}$ or $\mathrm{m}^{4}$.


Fig. 6.1

For a plane area composed of $n$ subareas $A_{i}$, each of whose moment of inertia is known about the $x$ - and $y$ - axes, the integral is replaced by a summation:

$$
\begin{equation*}
I_{x}=\sum_{i=1}^{n} I_{x_{i}}, \quad I_{y}=\sum_{i=1}^{n} I_{y_{i}} . \tag{6.1}
\end{equation*}
$$

The units of moment of inertia are the fourth power of a length, in ${ }^{4}$ or $\mathrm{m}^{4}$.

The parallel - axis theorem for moment of inertia of a finite area states that the moment of inertia of an area about any axis is equal to the moment of inertia about a parallel axis through the centroid of the area plus the product of the area and the square of the perpendicular distance between the two axes. For the area shown in Fig. 6.1, the axes $x$ and $y$ pass through the centroid of the plane area. The $x_{1}$ - and $y_{1}-$ axes are parallel axes located at distances $a$ and $b$ from the centroidal axes.

Let $A$ denote the area of the figure, $I_{x}$ and $I_{y}$ the moments of inertia about the axes through the centroid, and $I_{x_{1}}$ and $I_{y_{1}}$ the moments of inertia about the $x_{1}$ - and $y_{1}$-axes. Then we have:

$$
\begin{align*}
& I_{x_{1}}=I_{x}+a^{2} A  \tag{6.2}\\
& I_{y_{1}}=I_{y}+b^{2} A . \tag{6.3}
\end{align*}
$$

Derive the parallel - axis theorem for moments of inertia of a plane area.

Let us consider the plane area $A$ shown in Fig. 6.1. The axes $x$ and $y$ pass through its centroid, whose location is presumed to be known. The axes $x$ and $y$ are loca1ed at known distances $a$ and $b$, respectively, from the axes through the centroid.

For the clement of area do the moment of inertia about the $x_{1}-$ axis is given by:

$$
d I_{x_{1}}=(y+a)^{2} d A .
$$

For the entire area $A$ the moment of inertia about the $x_{1}$ - axis is:

$$
I_{x_{1}}=\int d I_{x_{1}}=\int_{A}(y+a)^{2} d A=\int_{A} y^{2} d A+\int_{A} a^{2} d A+2 a \int_{A} y d A .
$$

The second integral on the right is equal to:

$$
a^{2} \int_{A} d A=a^{2} A,
$$

because $a$ is a constant. The third integral on the right is equal to:

$$
2 a \int_{A} y d A=2 a \cdot 0=0,
$$

because the axis from which $y$ is measured passes through the centroid of the area. The first in1egral on the right is equal to $I_{x}$ i.e., the moment of inertia of the area about the horizontal axis through the centroid. Thus

$$
I_{x_{1}}=I_{x}+a^{2} A
$$

A similar considcra1ion in the other direction would show that:

$$
I_{y_{1}}=I_{y}+b^{2} A .
$$

This is the parallel - axis theorem for plane areas. It is to be noted that one of the axes involved in each equa1ion must pass through the centroid of the area. In words, this may be stated as follows: the moment of inertia of an area with reference to an axis not through the centroid of the area is equal to the moment of inertia about a parallel axis through the centroid of the area plus the product of the same area and the square of the distance between the two axes.

The moment of inertia always has a positive value with a minimum value for axes through the centroid of the area in question.

If the moment of inertia of an area $A$ about the $x_{1}$-axis is denoted by $I_{x_{1}}$. Then the radius of gyration $i_{x}$ is defined by:

$$
\begin{equation*}
i_{x}=\sqrt{\frac{I_{x}}{A}} . \tag{6.4}
\end{equation*}
$$

Similarly, the radius of gyration with respect to the $y$ - axis is given by:

$$
\begin{equation*}
i_{y}=\sqrt{\frac{I_{y}}{A}} . \tag{6.5}
\end{equation*}
$$

Since $I$ is in units of length to the fourth power, and $A$ is in units of length to the second power, then the radius of gyration has the units of length, say in or m . It is frequently useful for comparative purposes but has no physical significance.

The product of inertia of an element of area with respect to the $x-$ and $y$-axes in the plane of the area is given by:

$$
d I_{x y}=x y d A
$$

where $x$ and $y$ are coordinates of the elemental area as shown in Fig. 6.1.

The product of inertia of a finite area with respect to the $x$ - and $y$ - axes in the plane of the area is given by the summation of the products of inertia about those same axes of all elements of area contained within the finite area. Thus:

$$
\begin{equation*}
I_{x y}=\int x y d A . \tag{6.6}
\end{equation*}
$$

From this, it is evident that $I_{x y}$ may be positive, negative, or zero. For a plane area composed of $n$ subareas $A$, each of whose product of inertia is known with respect to specified $x$-and $y$-axes, the integral is replaced by the summation:

$$
\begin{equation*}
I_{x y}=\sum_{i=1}^{n} I_{x_{i} y_{i}} . \tag{6.7}
\end{equation*}
$$

The parallel - axis theorem for product of inertia of a finite area states that the product of inertia of an area with respect to the $x$ - and $y$ - axes is equal to the product of inertia about a set of parallel axes passing through the centroid of the area plus the product of the area and the two perpendicular distances from the centroid to the $x_{1}$ - and $y_{1}$ - axes. For the area, shown in Fig. 6.1, the axes $x$ and $y$ pass through the centroid of the plane area. The $x$ - and $y$ - axes are
parallel axes located at distances $x_{1}$ and $y_{1}$ from the centroidal axes. Let $A$ represent the area of the figure and $I_{x y}$ be the product of inertia about the axes through the centroid. Then we have:

$$
\begin{equation*}
I_{x_{1} y_{1}}=I_{x y}+a b A \tag{6.8}
\end{equation*}
$$

Let us derive the parallel - axis theorem for product of inertia of a plane area.

In Fig. 6.1 the axes $x$ and $y$ pass through the centroid of the area $A$. The axes $x_{1}$ and $y_{1}$ are located the known distances $a$ and $b$, respectively, from the axes through the centroid.

For the element of area do the product of inertia with respect to the $x_{1}$ - and $y_{1}$ - axes is given by:

$$
d I_{x_{1} y_{1}}=(y+a)(x+b) d x d y .
$$

For the entire area the product of inertia with respect to the $x_{1}-$ and $y_{1}$-axes becomes:

$$
I_{x_{1} y_{1}}=\int d I_{x_{1} y_{1}}=\iint_{A}(y+a)(x+b) d A=a b \int_{A} d A+a \int_{A} x d A+b \int_{A} y d A+\int_{A} x y d A .
$$

The first integral on the right side equals $a b A$ since $a$, and $b$ are constants. The second and third integrals vanish because $x$ and $y$ are measured from the axes through the centroid of the area $A$. The fourth integral is equal to $I_{x y}$, that is, the product of inertia of the area with respect to axes through its centroid and parallel to the $x_{1}-$ and $y_{1}-$ axes. Thus, we have:

$$
I_{x_{1} y_{1}}=I_{x y}+a b A
$$

This is the parallel - axis theorem for product of inertia of a plane area. It is to be noted that the $x$ - and $y \cdot$ - axes must pass through the centroid of the area. Also, $x_{1}$ - and $y_{1}$ arc positive only when the $x_{1}$ and $y_{1}$ - coordinates have the location relative to the $x y$ system indicated in Fig. 6.1. Thus, care must be taken with regard to the algebraic signs of $x$ and $y$.

## On beginning

### 6.2. Principal moments of inertia.

Let us consider a plane area A and assume that $I_{x}, I_{y}$ and $I_{x y}$ are known. Determine the moments of inertia $I_{u}$ and $I_{v}$ as well as the product of inertia $I_{u v}$ for the set of orthogonal axes $u, v$ oriented as shown in Fig. 6.2. Determine also the maximum and minimum values of $I_{u}$.


Fig. 6.2

The moment of inertia of the area with respect to the $u$-axis is:

$$
\begin{array}{r}
I_{u}=\int_{A}(-x \cdot \sin \alpha+y \cdot \cos \alpha)^{2} d A=\cos ^{2} \alpha \int_{A}^{2} d A+\sin ^{2} \alpha \int_{A}^{2} d A- \\
-\sin 2 \alpha \int_{A} x y d A=I_{x} \cos ^{2} \alpha+I_{y} \sin ^{2} \alpha-I_{x y} \sin 2 \alpha .
\end{array}
$$

Or

$$
\begin{equation*}
I_{u}=\frac{I_{x}+I_{y}}{2}+\frac{I_{x}-I_{y}}{2} \cos 2 \alpha-I_{x y} \sin 2 \alpha . \tag{6.9}
\end{equation*}
$$

Analogously, $I_{v}$ may be obtained from (6.9) by replacing $\alpha$ by $\alpha+\frac{\pi}{2}$ to yield:

$$
\begin{equation*}
I_{v}=\frac{I_{x}+I_{y}}{2}-\frac{I_{x}-I_{y}}{2} \cos 2 \alpha+I_{x y} \sin 2 \alpha \tag{6.10}
\end{equation*}
$$

The value of $\alpha$ that renders $I_{u}$, maximum or minimum is found by setting the derivative of Eq. (6.9) with respect to $\alpha$ equal to zero. Thus, since $I_{u}, I_{v}$ and $I_{u v}$ are constants we have from (6.9):

$$
\frac{d I_{u}}{d \alpha}=-\left(I_{x}-I_{y}\right) \sin 2 \alpha-2 I_{x y} \cos 2 \alpha=0
$$

Solving,

$$
\begin{equation*}
\operatorname{tg} 2 \alpha=\frac{2 I_{x y}}{\left(I_{y}-I_{x}\right)} \tag{6.11}
\end{equation*}
$$

If now the values of $2 \alpha$ given by (6.11) are substituted into (6.8), we obtain:

$$
\begin{equation*}
I_{u_{\max }}=\frac{I_{x}+I_{y}}{2} \pm \frac{1}{2} \sqrt{\left(I_{x}-I_{y}\right)^{2}+4 I_{x y}^{2}} \tag{6.12}
\end{equation*}
$$

where the positive sign refers to Case I and the negative sign to Case II. These maximum and minimum values of moment of inertia correspond to axes defined by (6.11). The maximum and minimum values of moment of inertia are termed principal moments of inertia and the corresponding axes arc termed principal axes.

We may now determine $I_{u v}$ from:

$$
\begin{aligned}
& I_{u v}=\int_{A}(x \cos \alpha+y \sin \alpha)(y \cos \alpha-x \sin \alpha) d A= \\
& =\cos ^{2} \alpha \int_{A} x y d A-\sin ^{2} \alpha \int_{A} x y d A+\cos \alpha \sin \alpha \int_{A}^{2} y^{2} d A+\cos \alpha \sin \alpha \int_{A}^{2} x^{2} d A=
\end{aligned}
$$

$$
\begin{equation*}
=\frac{I_{x}-I_{y}}{2} \sin 2 \alpha-I_{x y} \cos 2 \alpha . \tag{6.13}
\end{equation*}
$$

From (6.13), $I_{u v}$ vanishes if

$$
\operatorname{tg} 2 \alpha=\frac{2 I_{x y}}{\left(I_{y}-I_{x}\right)},
$$

which is identical to condition (6.11). Since (6.11) defined principal axes, it follows that the product of inertia vanishes for principal axes.

At any point in the plane of an area there exist two perpendicular axes about which the moments of inertia of the area are maximum and minimum for that point. These maximum and minimum values of moment of inertia are termed principal moments of inertia and are given by:

$$
\begin{align*}
& I_{\max }=\frac{I_{x}+I_{y}}{2}+\frac{1}{2} \sqrt{\left(I_{x}-I_{y}\right)^{2}+4 I_{x y}^{2}},  \tag{6.14}\\
& I_{\min }=\frac{I_{x}+I_{y}}{2}-\frac{1}{2} \sqrt{\left(I_{x}-I_{y}\right)^{2}+4 I_{x y}^{2}} .
\end{align*}
$$

The pair of perpendicular axes through a selected point about which the moments of inertia of a plane area are maximum and minimum are termed principal axes.

The product of inertia vanishes if the axes are principal axes. Also, from the integral defining product of inertia of a finite area, it is evident that if either the $x$-axis, or the $y$-axis, or both, are axes of symmetry, the product of inertia vanishes. Thus, axes of symmetry are principal axes.

## On beginning

### 5.3. Solved problem

Find basic geometrical characterizations for plane transversal section, represented on Fig. 6.3.

1. For finding of the centroid of the section and its geometrical characterizations will divide a section into three simple figures (Fig. 6.4): rectangle $K D K_{1} K_{2}$ and two triangles $K K_{2} B$ and $K_{1} D C$. These triangles are conveniently considered together.


Fig. 6.3
2. Areas of constituents of the section are levels:

$$
\begin{aligned}
& A_{K D K_{1} K_{2}}=0,4 \cdot 1=0,4 \mathrm{~cm}^{2}=0,4 \mathrm{~m}^{2} ; \\
& A_{K K_{2} B}=0,5 \cdot 0,2 \cdot 1=0,1 \mathrm{~cm}^{2}=0,1 \mathrm{~m}^{2} ; \\
& A_{K_{1} D C}=0,5 \cdot 0,2 \cdot 1=0,1 \mathrm{~cm}^{2}=0,1 \mathrm{~m}^{2} .
\end{aligned}
$$

Then a general area a section is equal:

$$
A=2 \cdot 0,1+0,4=0,6 \mathrm{~m}^{2}
$$

The section has a axis of symmetry $x$, and that is why determine the co-ordinate of centroid of $x_{c}$ only $\left(y_{c}=0\right)$.

$$
\begin{aligned}
& \text { As } x_{c_{1}}=0, x_{c_{2}}=x_{c_{2}}=-\frac{1}{6} \mathrm{~m}, \text { then: } \\
& x_{c}=\frac{x_{c_{1}} \cdot A_{1}+x_{c_{2}} \cdot\left(A_{2}+A_{3}\right)}{A}=\frac{0,4 \cdot 0+0,2 \cdot\left(-\frac{1}{6}\right)}{0,6}=-\frac{1}{18} \mathrm{~m} \approx-0,0556 \mathrm{~m} .
\end{aligned}
$$



Fig. 6.4
2. At the calculation of moments of inertia will take into account that for triangles moments of inertia about the axes of $x$ and $y$ are determined by formulas:

$$
I_{x}=\frac{b h^{3}}{36}, \quad I_{y}=\frac{b^{3} h}{36}
$$

and the centrifugal moment of inertia is accordingly equal:

$$
I_{x y}=-\frac{b^{2} h^{2}}{72}
$$

Then moments of inertia about central axes of section determined by formulas:

$$
\begin{aligned}
& I_{x_{c}}=\sum_{i=1}^{3}\left(I_{x_{c_{i}}}+y_{c_{i}}^{2} \cdot A_{i}\right)=\left(\frac{1 \cdot 0,4^{3}}{12}\right)+2\left(\frac{1 \cdot 0,2^{3}}{36}+\left(0,2+\frac{0,2}{3}\right)^{2} \cdot 0,1\right)= \\
& =0,005333+0,014667=0,02 \mathrm{~m}^{4} \\
& I_{y_{c}}=\sum_{i=1}^{3}\left(I_{y_{c_{i}}}+x_{c_{i}}^{2} \cdot A_{i}\right)=\left(\frac{1^{3} \cdot 0,4}{12}+\left(-\frac{1}{18}\right)^{2} \cdot 0,4\right)+
\end{aligned}
$$

$+2\left(\frac{1^{3} \cdot 0,2}{36}+\left(\frac{2}{18}\right)^{2} \cdot 0,1\right)=0,03456+0,01358=0,04814 \mathrm{~m}^{4}$.
As this section has an axis of symmetry $x_{C}$, the centrifugal moment of inertia about it is equal to the zero. And it means that axes $x_{C}$ and $y_{C}$ are principal central axes of given section. Accordingly, moments of inertia $I_{x_{C}}$ and $I_{y_{C}}$ are principal central moments of inertia for given transversal section

## On beginning

